Effects of Hydraulic Overload on Biological Nutrient Removal Efficiencies in Wastewater Treatment Systems

Thandeka Y.S. Jwara, Paul Musonge and Babatunde F. Bakare

Abstract— The control of eutrophication in receiving water bodies and adherence to the current wastewater treatment environmental laws, has led to extensive use of the biological nutrient removal process (BNR) in wastewater treatment practice. Wastewater treatment is a sensitive and complex phenomenon which requires careful design and operation. Results of a comprehensive study on BNR efficiencies in relation to the hydraulic load was conducted for Darvill Wastewater Treatment Plant (WWTP). The plant influent and effluent nutrients (ammonia and soluble reactive phosphorus (SRP) concentrations were determined using standard laboratory testing methods. Calculations of the BNR efficiencies were used to determine the effects of high hydraulic loads to the biological treatability of the activated sludge for the period September 2016-November 2017. At hydraulic loads above design capacity the BNR efficiency was found to be at an average of 40% and SRP removal efficiency being 64% for the period of the study. The BNR efficiency had an inversely proportional relationship to the hydraulic load into the plant with ammonia removal being mostly affected. When the plant is hydraulically overloaded the BNR process is adversely affected.

Keywords— Biological nutrient removal, hydraulically overloaded plants, nutrient removal efficiency, wastewater treatment

I. INTRODUCTION

Wastewater treatment has over the last decade become an essential step to improved recipient water quality, reduced water pathogenicity and improved human health. To control eutrophication in receiving water resources, biological nutrient removal (BNR) of nitrogen and phosphorus has been extensively used in wastewater treatment systems, both for the upgrade of existing wastewater treatment plants (WWTPs) and the design of such new facilities [1]. These WWTPs are further subject to comply with the national legislative requirements of the Department of Water and Sanitation (DWS) in order to preserve and protect the water resources that WWTPs discharge

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Babatunde F. Bakare is with the Department of Chemical Engineering, Mangosuthu University of Technology, Durban, South Africa. into. In the case of BNR, the activated sludge reactor is the most commonly used technology [2]. Traditionally, the complexity associated with implementing BNR in wastewater treatment systems has been largely in terms of balancing the competing requirements for nitrogen (N) and phosphorus (P) removal, particularly with respect to the influent readily biodegradable chemical oxygen demand (rbCOD) which becomes a substrate for the microorganisms [3].

Enhanced biological phosphorous removal (EBPR) employs the extended polyphosphate storage capacity of individual microorganisms known as the polyphosphate-accumulating organisms (PAOs). These PAOs require anaerobic conditions in order to assimilate organic matter such as volatile fatty acids (VFAs) with the release of phosphorus from stored polyphosphate [4]. Phosphate is taken up, under aerobic conditions, by the PAOs [5] and as has been demonstrated, under anoxic conditions in the presence of nitrate by denitrifying-polyphosphate-accumulating organisms (DPAOs) [3][6]. The DPAOs, in fact, denitrify nitrite and nitrate (NO^{x-}) using the organic matter previously stored under anaerobic conditions [6]. Therefore, DPAOs optimize denitrification, which can be limited by the low quantity of organic matter available in the anoxic reactor. Moreover, phosphate uptake occurs either under anoxic or aerobic conditions increasing phosphorus removal efficiencies [7].

In the case of N removal, typically, the nitrogen (N) found in the wastewater treatment system is present in the of NH₃, ammonium ions (NH⁴⁺), nitrites (NO²⁻) and nitrates (NO³⁻). Ammonia and Ammonium ions are frequently referred to as "ammonia nitrogen". Ammonia and ammonium ions are the most reduced forms of N and are often products of organic decomposition. Nitrite is, however, the intermediary, with nitrate being the end product of organic oxidation during nitrification [2]. The removal of ammonia nitrogen in wastewater through nitrification is mediated by specific chemical autotrophic nitrifying bacteria. [6] suggests that these bacteria obtain their carbon requirement (anabolism) from dissolved carbon dioxide (CO₂) and their energy requirement (catabolism) for biomass synthesis from oxidizing the ammonia nitrogen to nitrite and nitrate. In the activated sludge reactor, the autotrophic nitrifying bacteria have considerably lower growth

coefficients (1/5th) as compared to the heterotrophic bacteria which are responsible for chemical oxygen demand (COD) removal.

Industrial effluent is observed to have high concentration of N and a low C/N ratio. The main difficulties in maintaining high nitrification efficiency when treating low C/N wastewaters are changes in influent concentration and flow, which may also affect the dissolved oxygen level in the reactor, and pH due to fluctuating industrial operations [8].

The operating WWTP used for this study is Darvill WWTP. The WWTP is situated in Pietermaritzburg and owned by Umgeni Water. The plant is operated on the basic design of the conventional activated sludge configuration which consists of primary settling tanks (PST), activated sludge reactor, secondary settling tanks (SST) and chlorine disinfection of final effluent. The WWTP discharges into the Umsunduzi River. The discharge limits for NH₃ and SRP are 6 mg N/l and 1 mg P/l respectively. It has an average dry weather flow (ADWF) design capacity of 65 Ml/d; however, the WWW has been receiving wastewater influent flow as high as 120 Ml/d for the period of September 2016-November 2017. Darvill WWTP had a challenge with nutrient removal through this period thus compromising the WWTP legal compliance. The study seeks to analyze the effects of the high influent flows (hydraulic overload) on the BNR process. This analysis study is particularly observing impacts on the nutrients, ammonia and SRP.

II. METHOD

Darvill wastewater works was used as a case study to conduct this study. The main focus area is the performance of the activated sludge reactor when subjected to flows above its design capacity of 65 Ml/d.

Sampling

A one litre representative grab sample of primary effluent and final effluent was collected and appropriately labelled every morning at 8:00 am. Primary effluent is the supernatant from the primary settling tank that goes into the activated sludge reactor before BNR. The collection times and sampling points were kept constant throughout the period of the study. Once the samples were collected they were stored in a cooler box with ice and transported immediately to the laboratory for analysis and were handled within three days after drop-off.

Laboratory Analysis

The ammonia and SRP concentrations were determined as outlined by the Standard Methods (1995) for ammonia and SRP determination. The samples were centrifuged at 5000 rpm for 5 minutes (Megafuge 3.0 R) and the clear supernatants were analyzed accordingly. The DR900 Spectrophotometer instrument was used to determine the concentration of NH $_3$ and SRP.

Statistical Analysis and calculations

The concentrations from the lab analyses were used to calculate the BNR efficiency of each parameter in the primary effluent and final effluent sample. The plant inflow was recorded hourly and an average for the day was recorded.

$$Eff = \frac{PEc - FEc}{PEc} \tag{1}$$

Where:

Eff – Removal efficiency

PEc – Primary effluent concentration

FEc – Final effluent concentration

Microsoft Excel, was used for the scatter plots to compare the BNR efficiencies to the plant inflow.

III. RESULTS

The plant flows for 2016-2017 were plotted and compared to the removal efficiencies of ammonia and SRP. This was done to establish the effects of hydraulic overloading on the BNR process of Darvill WWTP.

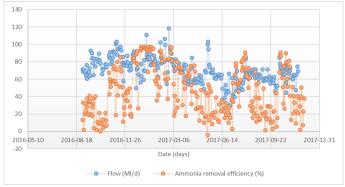


Fig. 1 Ammonia removal efficiency

The graph (see Fig. 1) suggests that the NH₃ removal efficiency is mainly inversely proportional to the plant influent flows. The removal efficiency average of ammonia during the period of September 2016 and November 2017 was found to be 42% and 38% respectively.

The design capacity of Darvill WWTP is 65 Ml/d, however, the data shows numerous periods where the plant was operating above the design capacity, experiencing peaks as high as 120 Ml/d. The lowest ammonia removal efficiency was seen in June 2017 (23.2%) with a flow of 48.9 Ml/d and November 2016 on

average had the highest influent flows (88Ml/d). This was largely associated with procurement delays of faulty subsurface aerators which resulted in process upsets. This was also suspected to be as a result of the cold weather temperatures experienced by the WWTP during winter which could reach a minimum of 5 $^{\circ}$ C. The denitrification rate is significantly affected by lower temperatures in wastewater treatment systems and the denitrification rate decreases 10 times linearly as compared to temperatures of 20 $^{\circ}$ C and above [9] [10] [11]. These temperature effects on BNR are predominantly related to the low growth rate of the nitrifying bacteria at low temperatures [12]. A study [13] found that Nitrogen removal at low temperatures could be achieved by operating the WWTP at higher mixed liquor suspended solids (MLSS) concentration of the range 6000-8000 mg/l and an adequate supply of COD for denitrification. This however could not be practiced at Darvill WWTP as the design MLSS concentration was 4500 mg/l and higher MLSS concentrations led to other process challenges i.e. sludge bulking. Wastewater treatability depends largely on the characteristics of the sludge and the activated sludge reactor conditions [6]. Higher influent flows come with higher organic and nutrient loads hence compromising the BNR process. It was seen that for the most part of the months of the study the ammonia removal efficiency was below 75% (see

Table 1) this shows that the Darvill WWW treatment process was negatively affected by the hydraulic overload.

TABLE I AVERAGE MONTHLY INFLUENT FLOW AND AMMONIA REMOVAL EFFICIENCY DATA

Month/Yea r	Influent Flow average	Removal efficiency average
Sep-16	73,7	24,1
Oct-16	79,3	27,6
Nov-16	88	51,9
Dec-16	75	63
Jan-17	81,1	87
Feb-17	84,5	68,9
Mar-17	74,7	55,1
Apr-17	60,9	50,2
May-17	70,8	25,7
Jun-17	48,9	23,2
Jul-17	52,2	59,2
Aug-17	66,4	42,3
Sep-17	60,8	19,8
Oct-17	64,9	60
Nov-17	64,2	29

The average SRP removal efficiency during the period of 2016 and 2017 was observed to be 61.6% and 65.9% for the two years respectively. There is an inversely proportional relationship (see Fig. 2). The results suggest that at higher influent flows more detrimental effect are observed on the ammonia removal efficiency than that SRP. This is mainly due to the fact that ammonia is removed during the nitrification process in the activated sludge reactor. The amount of dissolved oxygen available in the activated sludge reactor becomes insufficient and the growth of the autotrophic nitrifying bacteria becomes compromised at flows above design capacity. There is also a significant increase in organic loading which also requires and competes for the available dissolved oxygen in the activated sludge reactor. The POAs are able to compete successfully with the other microorganisms for substrate in completely aerobic activated sludge systems [14]. The decline of the SRP removal efficiency is largely due to the wash out of POA's with the final effluent in instances of hydraulic overload. [15] suggested that WWTPs experience reduced SRP removal rates at low temperatures due to reduced biological reaction rates. However, Darvill WWTP SRP removal efficiency was not significantly adversely affected by the low temperatures. Any process upsets can also affect the performance of the microorganisms in the activated sludge reactor. Therefore, monitoring and regulation of online parameters such as pH, DO, SRP and nutrient pollutants (phosphorus and NH⁴⁺–N/NO²⁻ –N /NO³⁻ –N) can be helpful in achieving and maintaining high nutrient removal in wastewater treatment systems [16].

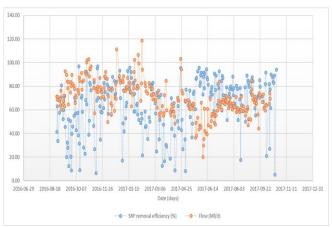


Fig. 2 SRP removal efficiency

TARIEIIA	VERAGE MONTHLY INFLOW	AND SRP DEMOVAL	EFFICIENCIES DATA

Month/Year	Influent flow average (MI/d)	Removal efficiency average (%)
Sep-16	74,7	47,4
Oct-16	84,0	56,6
Nov-16	80,3	64,4
Dec-16	76,5	77,8
Jan-17	80,9	69,0
Feb-17	84,6	54,3
Mar-17	66,3	52,0
Apr-17	67,8	49,1
May-17	57,3	66,6
Jun-17	49,0	80,8
Jul-17	66,3	74,0
Aug-17	60,5	69,1
Sep-17	66,5	73,0
Oct-17	69,3	70,6

The average monthly influent flow and SRP removal efficiencies data (see Table 2) shows that the highest SRP removal (80.8%) was observed in June 2017 at a low influent flow of 49 Ml/d. This shows that the process upset during this time had little to no effect on the POA's ability to uptake the phosphorus in the activated sludge reactor. This is mainly due to the fact that POAs are facultative microorganisms which means they can survive in presence and absence of oxygen.

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